## ANALYSES OF DELAMINATIONS IN NON-LINEAR ELASTIC MULTILAYERED INHOMOGENEOUS BEAM CONFIGURATIONS

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#### **ARTICLE INFO** Abstract: Article history: This paper presents investigation of delamination Received: 11.7.2018. fracture behavior of multilayered non-linear elastic Received in revised form: 8.11.2018. beam configurations by using the Ramberg-Osgood Accepted: 22.11.2018. stress-strain relation. It is assumed that each layer Keywords: exhibits continuous material inhomogeneity along Inhomogeneous material the width as well as along thickness of the layer. An Delamination approach for determination of the strain energy Material non-linearity release rate is developed for a delamination crack located arbitrary along the multilayered beam J-integral height. The approach can be applied for multilayered Strain energy release rate *Four-point bending* beams of arbitrary cross-section under combination of axial force and bending moments. The layers may Multilayered beam Complementary strain energy have different thickness and material properties. The number of layers is arbitrary. The approach is DOI: http://doi.org/10.30765/er.40.3.07 applied for analyzing the delamination fracture behavior of a multilayered beam configuration subjected to four-point bending. The beam has a rectangular cross-section. The delamination crack is located symmetrically with respect to the beam midspan. The strain energy release rate is derived assuming that the modulus of elasticity varies continuously in the cross-section of each layer according to a hyperbolic law. In order to verify the solution to the strain energy release rate, the delamination fracture behavior of the multilayered non-linear elastic four-point bending beam configuration is studied also by applying the method of the J-integral. The solution to the strain energy release rate derived in the present paper is used in order to perform a parametric study of delamination.

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### 1 Introduction

Inhomogeneous structural materials exhibit varying material properties in the volume of the solid. Since the material properties are functions of the coordinates, the analysis of fracture in structural members and components made of inhomogeneous materials is a challenging problem. Nevertheless, the development of fracture analyses is an important task of the fracture mechanics of inhomogeneous materials because these analyses give useful information about the influence of various factors such as geometry of the structural member, crack location, material response and others on the fracture behaviour. This information can be significant in order to better understand the fracture phenomenon. Also, the knowledge gathered through these analyses can be used in the design of inhomogeneous structural components in order to improve and optimize their fracture performance.

One of the reasons for the increased interest towards the inhomogeneous materials is that the functionally materials, which are а class graded of inhomogeneous materials, have been widely applied in practical engineering [1-12]. Functionally graded materials are made by mixing two or more constituent materials. Due to the variation of material properties along one or more spatial coordinates, the microstructure of functionally graded materials can be tailored during manufacturing. This fact makes these new inhomogeneous materials an attractive alternative to the conventional structural materials in various applications in aerospace systems, aircrafts, power plants, microelectronics nuclear and biomedicine.

Multilayered materials are a kind of inhomogeneous materials which are made of adhesively bonded layers of different materials. The concept of multilayered materials opens excellent opportunities for adequate utilization of strength of different materials assembled in one structure and provides an effective mean for optimization of structural performance. Thus, the multilayered structures have a high load-bearing capacity and are usually much more economical in comparison with their metal counterparts. Since the stiffness-to-weight and strength-to-weight ratios of multilayered materials are high, significant weight savings can be realized. Therefore, multilayered materials are very suitable for light structures in various engineering applications where low weight is particularly important.

One of the major concerns in the use of multilayered materials and structures in practical engineering is their delamination fracture behaviour. The delamination phenomenon involves longitudinal fracture, i.e. separation of adjacent layers. In fact, the delamination is the most often type of failure occurring in multilayered structures. The safety and durability of multilayered structural members and components depend strongly upon their delamination behaviour. Usually, fracture in multilayered structural components has been studied by the methods of linear-elastic fracture mechanics assuming linear-elastic behaviour of the material [13, 141.

Recently, particular delamination fracture analyses of non-linear elastic multilayered beam configurations of rectangular cross-section loaded in bending have been developed assuming that each layer exhibits material inhomogeneity [15-17]. The material nonlinearity has been modeled mainly by using powerlaw stress-strain relations.

The delamination fracture behaviour of a non-linear elastic multilayered beam structure of rectangular cross-section has been studied in terms of the strain energy release rate [17]. The beam is made of vertical bonded longitudinal adhesively layers. Α delamination crack is located arbitrary between layers. It has been assumed that the layers exhibit material inhomogeneity in the thickness direction. The beam is loaded in bending about the horizontal centroidal axis of the cross-section. The non-linear mechanical behaviour of the material has been treated by using a power-law stress-strain relation. A solution to the strain energy release rate has been derived assuming that the coefficient in the powerlaw varies continuously along the thickness of layers. This paper presents a general approach for determination of the strain energy release rate for delamination cracks in multilayered non-linear elastic beams is developed by using the Ramberg-Osgood stress-strain relationship for modeling the material non-linearity in contrast to previous papers [15-17] which are concerned with particular solutions of the strain energy release rate for separate beam configurations. Multilayered beams of arbitrary cross-section are considered in the present paper. The beams are made of arbitrary number of adhesively bonded horizontal layers. Each layer has individual thickness and material properties. Besides, it is assumed that each layer exhibits smooth material inhomogeneity along the width as well as along thickness of layer. The delamination crack is located arbitrary between layers. The beams considered are under combination of axial force and bending moments. The approach developed in the present paper is applied for studying the strain energy release rate for a delamination crack in a multilayered nonlinear elastic inhomogeneous beam configuration subjected to four-point bending. The method of *J*integral is used also for analysis of the delamination fracture behaviour of the multilayered four-point bending beam in order to verify the solution to the strain energy release rate.

# 2 Approach for determination of the strain energy release rate

A beam portion with the delamination crack front is shown schematically in Fig. 1.



Figure 1. Portion of a multilayered beam with the delamination crack front  $(b_S \text{ is the beam width at the delamination crack level})$ 

The heights of the lower and upper crack arms are denoted by  $h_1$  and  $h_2$ , respectively. The non-linear mechanical behaviour of the inhomogeneous material in the *i*-th layer of the beam is described by the Ramberg-Osgood stress-strain relationship

$$\varepsilon = \frac{\sigma_i}{E_i} + \left(\frac{\sigma_i}{H_i}\right)^{\frac{1}{m_i}}, i = 1, 2, ..., n$$
(1)

where *n* is the number of layers in the beam,  $\varepsilon$  is the longitudinal strain,  $\sigma_i$  is the longitudinal normal stresses,  $E_i$  is the modulus of elasticity,  $H_i$  and  $m_i$  are material properties in the *i*-th layer. It is assumed that the modulus of elasticity varies continuously in the cross-section of each layer

$$E_i = E_i(y_4, z_4), \ i = 1, 2, ..., n$$
 (2)

where  $y_4$  and  $z_4$  are the centroidal axes of the beam cross-section (Fig. 1).

The delamination fracture behaviour is analyzed in terms of the strain energy release rate, G. By applying the solution to the strain energy release rate derived in [18], the strain energy release rate for the multilayered beam in Fig. 1 can be written as

$$G = \frac{1}{b_S} \begin{pmatrix} \sum_{i=1}^{i=n_1} \iint_{(A)} u_{01i}^* dA + \\ \sum_{i=1}^{i=n_2} \iint_{(A)} u_{02i}^* dA - \sum_{i=1}^n \iint_{(A)} u_{0i}^* dA \end{pmatrix}$$
(3)

where  $b_S$  is the beam width at the level of the delamination crack,  $n_1$  and  $n_2$  are, respectively, the number of layers in the lower and upper crack arms,  $u_{01i}^*$ ,  $u_{02i}^*$  and  $u_{0i}^*$  are, respectively, the complementary strain energy densities in the *i*-th layer in the cross-section of the lower and upper crack arms behind the crack front and in the beam cross-section ahead of the crack front.



Figure 2. Cross-section of the lower and upper crack arms behind the crack front

The complementary strain energy density in the *i*-th layer of the cross-section of lower crack arm behind the crack front can be calculated by the following formula [19]:

$$u_{01i}^{*} = \frac{\sigma_i^2}{2E_i} + \frac{m_i \sigma_i^{\frac{1+m_i}{m_i}}}{(1+m_i)H^{\frac{1}{m_i}}}$$
(4)

In order to perform the integration of  $u_{01i}^*$  in (3),  $u_{01i}^*$  in (4) has to be expressed as a function of the coordinates,  $y_1$  and  $z_1$ , (Fig. 2). Apparently, the normal stress,  $\sigma_i$ , cannot be determined explicitly from the equation of Ramberg-Osgood (1). Therefore,  $\sigma_i$  is expanded in series of Taylor by keeping the first six members

$$\sigma_{i}(y_{1}, z_{1}) \approx \sigma_{i}(0, z_{1ai}) + \frac{\partial \sigma_{i}(0, z_{1ai})}{\partial y_{1}} y_{1} + \frac{\partial \sigma_{i}(0, z_{1ai})}{\partial z_{1}} (z_{1} - z_{1ai}) + \frac{\partial^{2} \sigma_{i}(0, z_{1ai})}{2! \partial y_{1}^{2}} y_{1}^{2} + \frac{\partial^{2} \sigma_{i}(0, z_{1ai})}{\partial y_{1} \partial z_{1}} y_{1}(z_{1} - z_{1ai}) + \frac{\partial^{2} \sigma_{i}(0, z_{1ai})}{2! \partial z_{1}^{2}} (z_{1} - z_{1ai})^{2}$$
(5)

where  $y_1$  and  $z_1$  are the centroidal axes of the lower crack arm cross-section (Fig. 2). In (5),

$$z_{1ai} = \frac{z_{1i} + z_{1i+1}}{2} \tag{6}$$

where  $z_{1i}$  and  $z_{1i+1}$  are the  $z_1$ -coordinates, respectively, of the upper and lower surfaces of the *i*-th layer.

Formula (5) is re-written as

$$\sigma_{i}(y_{1}, z_{1}) \approx v_{1i} + v_{2i}y_{1} + v_{3i}(z_{1} - z_{1ai}) + v_{4i}y_{1}^{2} + v_{5i}y_{1}(z_{1} - z_{1ai}) + v_{6i}(z_{1} - z_{1ai})^{2}$$
(7)

where the coefficients,  $v_{1i}$ ,  $v_{2i}$ ,  $v_{3i}$ ,  $v_{4i}$ ,  $v_{5i}$  and  $v_{6i}$ , are determined by applying the Ramberg-Osgood equation (1). For this purpose, by using the Bernoulli's hypothesis for plane sections, the strains in the lower crack arm cross-section are written as

$$\varepsilon = \varepsilon_{C_1} + \kappa_{y_1} y_1 + \kappa_{z_1} z_1 \tag{8}$$

where  $\varepsilon_{C_1}$  is the strain in the centre of the lower crack arm cross-section,  $\kappa_{y_1}$  and  $\kappa_{z_1}$  are the curvatures of lower crack arm in the  $x_1y_1$  and  $x_1z_1$ planes, respectively. The applicability of the Bernoulli's hypothesis is based on the fact that beams of high span to height ratio are analyzed in the present study. By substituting of (7) and (8) in (1), one obtains

$$\begin{split} \varepsilon_{C_{1}} + \kappa_{y_{1}} y_{1} + \kappa_{Z_{1}} z_{1} &= \\ \frac{1}{E_{i}} \Big[ \nu_{1i} + \nu_{2i} y_{1} + \nu_{3i} (z_{1} - z_{1ai}) + \nu_{4i} y_{1}^{2} + \\ \nu_{5i} y_{1} (z_{1} - z_{1ai}) + \nu_{6i} (z_{1} - z_{1ai})^{2} \Big] + \\ + \frac{1}{H_{i}^{\frac{1}{m_{i}}}} \Big[ \nu_{1i} + \nu_{2i} y_{1} + \nu_{3i} (z_{1} - z_{1ai}) + \nu_{4i} y_{1}^{2} + \\ H_{i}^{\frac{1}{m_{i}}} \Big] + \\ \nu_{5i} y_{1} (z_{1} - z_{1ai}) + \nu_{6i} (z_{1} - z_{1ai})^{2} \Big]^{\frac{1}{m_{i}}} \end{split}$$

At  $y_1 = 0$  and  $z_1 = z_{1ai}$ , formula (9) transforms in

$$\varepsilon_{C_1} + \kappa_{Z_1} z_{1ai} = \frac{v_{1i}}{E_i} + \frac{v_{1i}^{\frac{m_i}{m_i}}}{H_i^{\frac{m_i}{m_i}}}$$
(10)

By substituting of  $y_1 = 0$  and  $z_1 = z_{1ai}$  in the first derivatives of (9) with respect to  $y_1$  and  $z_1$ , one arrives at

$$\kappa_{y_1} E_i + \varepsilon_{C_1} \frac{\partial E_i}{\partial y_1} = v_{2i} + \frac{1}{H_i^{\frac{1}{m_i}}} \left( \frac{\partial E_i}{\partial y_1} v_{1i}^{\frac{1}{m_i}} + E_i \frac{1}{m_i} v_{1i}^{\frac{1-m_i}{m_i}} v_{2i} \right)$$
(11)

$$\kappa_{z_1} E_i + \varepsilon_{C_1} \frac{\partial E_i}{\partial z_1} = v_{3i} + \frac{1}{H_i^{\frac{1}{m_i}}} \left( \frac{\partial E_i}{\partial z_1} v_{1i}^{\frac{1}{m_i}} + E_i \frac{1}{m_i} v_{1i}^{\frac{1-m_i}{m_i}} v_{3i} \right)$$
(12)

Further, by substituting of  $y_1 = 0$  and  $z_1 = z_{1ai}$  in the second derivatives of (9) with respect to  $y_1$  and  $z_1$  one obtains

$$2\kappa_{y_{1}} \frac{\partial E_{i}}{\partial y_{1}} + \varepsilon_{C_{1}} \frac{\partial^{2} E_{i}}{\partial y_{1}^{2}} = 2v_{4i}$$

$$+ \frac{1}{H_{i}^{\frac{1}{m_{i}}}} \left[ \frac{\partial^{2} E_{i}}{\partial y_{1}^{2}} v_{1i}^{\frac{1}{m_{i}}} + \frac{\partial E_{i}}{\partial y_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{2i} \right]$$

$$+ \left( \frac{\partial E_{i}}{\partial y_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + E_{i} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{2i} \right) v_{2i}$$

$$+ E_{i} \frac{2}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{4i} \right]$$

$$\kappa_{y_{1}} \frac{\partial E_{i}}{\partial z_{1}} + \kappa_{z_{1}} \frac{\partial E_{i}}{\partial y_{1}} + \varepsilon_{C_{1}} \frac{\partial^{2} E_{i}}{\partial y_{1} \partial z_{1}} =$$

$$= v_{5i} + \frac{1}{H_{i}^{\frac{1}{m_{i}}}} \left[ \frac{\partial^{2} E_{i}}{\partial y_{1} \partial z_{1}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + \frac{\partial E_{i}}{\partial y_{1} \partial z_{1}} \frac{1-m_{i}}{m_{i}} v_{3i} \right]$$

$$+ \left( \frac{\partial E_{i}}{\partial z_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + E_{i} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-2m_{i}}{m_{i}}} v_{3i} \right) v_{2i}$$

$$+ E_{i} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + E_{i} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right) v_{2i}$$

$$+ \left( \frac{\partial E_{i}}{\partial z_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + E_{i} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right) v_{2i}$$

$$+ \left( \frac{\partial E_{i}}{\partial z_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + \frac{\partial E_{i}}{\partial z_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right) v_{3i}$$

$$+ \left( \frac{\partial E_{i}}{\partial z_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + E_{i} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right) v_{3i}$$

$$+ \left( \frac{\partial E_{i}}{\partial z_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + E_{i} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right) v_{3i}$$

$$+ \left( \frac{\partial E_{i}}{\partial z_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + E_{i} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right) v_{3i}$$

$$+ \left( \frac{\partial E_{i}}{\partial z_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + E_{i} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right) v_{3i}$$

$$+ \left( \frac{\partial E_{i}}{\partial z_{1}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{2i} \right) v_{3i}$$

It should be specified that the modulus of elasticity and its derivatives,  $\frac{\partial E_i}{\partial y_1}$ ,  $\frac{\partial E_i}{\partial z_1}$ ,  $\frac{\partial^2 E_i}{\partial y_1^2}$ ,  $\frac{\partial^2 E_i}{\partial y_1 \partial z_1}$  and  $\frac{\partial^2 E_i}{\partial z_1}$ , which participate in equations (10) (15)

 $\frac{\partial^2 E_i}{\partial z_1^2}$ , which participate in equations (10) – (15)

have to be calculated at  $y_1 = 0$  and  $z_1 = z_{1ai}$ .

Equations (10) – (15) can be written for each layer in the lower crack arm. In this way,  $6n_1$  equations with  $6n_1 + 3$  unknowns,  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$ ,  $\kappa_{z_1}$  and  $v_{1i}$ ,  $v_{2i}$ ,  $v_{3i}$ ,  $v_{4i}$ ,  $v_{5i}$ ,  $v_{6i}$  where  $i = 1, 2, ..., n_1$ , can be obtained. Three other equations can be written by considering the equilibrium of the lower crack arm cross-section which is under combination of axial force and bending moments (Fig. 2). The equilibrium equations are expressed as

$$N_1 = \sum_{i=1}^{i=n_1} \iint_{(A)} \sigma_i dA \tag{16}$$

$$M_{y_{1}} = \sum_{i=1}^{i=n_{1}} \iint_{(A)} \sigma_{i} z_{1} dA$$
(17)

$$M_{z_1} = \sum_{i=1}^{i=n_1} \iint_{(A)} \sigma_i y_1 dA$$
 (18)

where  $\sigma_i$  is found by (7). In (16) – (18),  $N_1$  is the axial force,  $M_{y_1}$  and  $M_{z_1}$  are the bending moments about the centroidal axes,  $y_1$  and  $z_1$ . Equations (10) – (18) should be solved with respect to  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$ ,  $\kappa_{z_1}$ ,  $v_{1i}$ ,  $v_{2i}$ ,  $v_{3i}$ ,  $v_{4i}$ ,  $v_{5i}$  and  $v_{6i}$  where  $i = 1, 2, ..., n_1$ , by using the MATLAB R2013a computer program for particular beam structure and material properties. Then,  $u_{01i}^*$  where  $i = 1, 2, ..., n_1$ 

The complementary strain energy density in the *i*-th layer of upper crack arm behind the crack front can be calculated using formula (4). For this purpose,  $\sigma_i$  has to be replaced with  $\sigma_{gi}$  where  $\sigma_{gi}$  is the normal stress in the *i*-th layer of upper crack arm. Besides,  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$ ,  $\kappa_{z_1}$ ,  $v_{1i}$ ,  $v_{2i}$ ,  $v_{3i}$ ,  $v_{4i}$ ,  $v_{5i}$ ,  $v_{6i}$ ,  $n_1$ ,  $N_1$ ,  $M_{y_1}$  and  $M_{z_1}$  have to be replaced, respectively, with  $\varepsilon_{C_2}$ ,  $\kappa_{y_2}$ ,  $\kappa_{z_2}$ ,  $v_{g1i}$ ,  $v_{g2i}$ ,  $v_{g3i}$ ,  $v_{g4i}$ ,  $v_{g5i}$ ,  $v_{g6i}$ ,  $n_2$ ,  $N_2$ ,  $M_{y_2}$  and  $M_{z_2}$  in equations (10) – (18). Here,  $\varepsilon_{C_2}$ ,  $\kappa_{y_2}$  and  $\kappa_{z_2}$  are, respectively, the strain in the centre of the upper crack arm crosssection, and the curvatures in the  $x_2y_2$  and  $x_2z_2$  planes,  $N_2$ ,  $M_{y_2}$  and  $M_{z_2}$  are the axial force and the bending moments (Fig. 2). After solving

equations (10) – (18) with respect to  $\varepsilon_{C_2}$ ,  $\kappa_{y_2}$ ,  $\kappa_{z_2}$ ,  $v_{g1i}$ ,  $v_{g2i}$ ,  $v_{g3i}$ ,  $v_{g4i}$ ,  $v_{g5i}$  and  $v_{g6i}$ , the normal stress,  $\sigma_{gi}$ , can be expressed by (7) and then substituted in (4).

Formula (4) can also be applied to calculate  $u_{0i}^*$  by replacing of  $\sigma_i$  with  $\sigma_{ri}$ . The normal stress,  $\sigma_{ri}$ , in the *i*-th layer of the beam cross-section ahead of the crack front can be expressed by replacing of  $v_{1i}$ ,  $v_{2i}$ ,  $v_{3i}$ ,  $v_{4i}$ ,  $v_{5i}$ ,  $v_{6i}$ ,  $z_{1ai}$ ,  $y_1$  and  $z_1$ , respectively, with  $v_{r1i}$ ,  $v_{r2i}$ ,  $v_{r3i}$ ,  $v_{r4i}$ ,  $v_{r5i}$ ,  $v_{r6i}$ ,  $z_{3ai}$ ,  $y_3$  and  $z_3$  in (7). Here,  $y_3$  and  $z_3$  are the centroidal axes of the beam cross-section ahead of the crack front (Fig. 3).



Figure 3. Beam cross-section ahead of the crack front

Also,  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$ ,  $\kappa_{z_1}$ ,  $v_{1i}$ ,  $v_{2i}$ ,  $v_{3i}$ ,  $v_{4i}$ ,  $v_{5i}$ ,  $v_{6i}$ ,  $n_1$ ,  $N_1$ ,  $M_{y_1}$  and  $M_{z_1}$  are replaced, respectively, with  $\varepsilon_{C_3}$ ,  $\kappa_{y_3}$ ,  $\kappa_{z_3}$ ,  $v_{r1i}$ ,  $v_{r2i}$ ,  $v_{r3i}$ ,  $v_{r4i}$ ,  $v_{r5i}$ ,  $v_{r6i}$ , n,  $N_3$ ,  $M_{y_3}$  and  $M_{z_3}$  in equations (10) – (18). After substituting of  $u_{01i}^*$ ,  $u_{02i}^*$  and  $u_{0i}^*$  in (3), the strain energy release rate can be calculated for given beam geometry, loading and material properties (the integration in (3) should be performed by using the MATLAB R2013a computer program).

### **3** Numerical results

In this section, numerical results are presented in order to illustrate the capabilities of the approach for determination of the strain energy release rate developed in section 2 of this paper. Delamination fracture behaviour of a multilayered inhomogeneous beam configuration of rectangular cross-section is investigated. The beam is shown schematically in Fig. 4.



Figure 4. Four-point bending multilayered beam configuration

The cross-section of the beam has width, b, and height, 2h. The beam is subjected to four-point bending. The external loading consists of two vertical forces, F, applied at the end sections of the beam. A vertical notch of depth  $h_2$  is introduced under the upper beam surface in order to generate conditions for delamination fracture. It is assumed that a delamination crack of length 2a is located symmetrically with respect to the beam mid-span. The heights of the lower and upper crack arms are denoted by  $h_1$  and  $h_2$ , respectively. It is obvious that the upper crack arm is free of stresses. The delamination crack is located in the middle beam portion,  $D_1D_2$ , that is loaded in pure bending. Due to the symmetry, only half of the beam,  $l_1 + l_2 \le x_4 \le 2(l_1 + l_2)$ , is analyzed (Fig. 4).

The delamination fracture is studied in terms of the strain energy release rate by using formula (3). For this purpose, first, the complementary strain energy density in the *i*-th layer of the lower crack arm behind the crack front is calculated by applying (4). It is assumed that the modulus of elasticity varies

continuously throughout the cross-section of the *i*-th layer according to the following hyperbolic law:

$$E_{i} = \frac{E_{fi}}{2} \left[ \frac{1}{1+q_{i}} \frac{\frac{b}{2}+y_{1}}{b} + \frac{1}{1+r_{i}} \frac{z_{1}-z_{1i}}{z_{1i+1}-z_{1i}} \right]$$
(19)

where

$$-\frac{b}{2} \le y_1 \le \frac{b}{2} \tag{20}$$

$$z_{1i} \le z_i \le z_{1i+1} \tag{21}$$

In (19),  $E_{fi}$  is the value of the modulus of elasticity in the upper left-hand vertex of the *i*-th layer crosssection,  $q_i$  and  $r_i$  are material properties which control the material inhomogeneity, respectively, in width and thickness direction of the layer,  $z_{1i}$  and  $z_{1i+1}$  are the coordinates, respectively, of the upper

and lower surfaces of the *i*-th layer in the lower crack arm (Fig. 5).



Figire 5. Cross-section of the lower crack arm of the four-point bending multilayered beam

By substituting (19) in equations (10) - (15), one obtains the following expression:

$$\varepsilon_{C_1} + \kappa_{Z_1} z_{1ai} = \frac{v_{1i}}{E_{fi_i} \psi_i} + \frac{v_{1i}^{\frac{1}{m_i}}}{H_i^{\frac{1}{m_i}}}$$
(22)

$$\kappa_{y_{1}}E_{fi}\psi_{i} - 2\varepsilon_{C_{1}}\frac{E_{fi}q_{i}}{b(2+q_{i})^{2}} = v_{2i} + \frac{1}{H_{i}^{\frac{1}{m_{i}}}} \left( -2\frac{E_{fi}q_{i}}{b(2+q_{i})^{2}}v_{1i}^{\frac{1}{m_{i}}} + \frac{1}{H_{i}^{\frac{1}{m_{i}}}} + E_{fi}\psi_{i}\frac{1}{m_{i}}v_{1i}^{\frac{1-m_{i}}{m_{i}}}v_{2i} \right)$$

$$E_{fi}r_{i}\zeta_{i}$$

$$(23)$$

$$\kappa_{z_{1}} E_{fi} \psi_{i} - \varepsilon_{C_{1}} \frac{g_{i} + g_{i}}{2\theta_{i}^{2}} = v_{3i} + \frac{1}{\frac{1}{m_{i}}} \left( -\frac{E_{fi} r_{i} \zeta_{i}}{2\theta_{i}^{2}} v_{1i}^{\frac{1}{m_{i}}} + E_{fi} \psi_{i} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right)$$
(24)

$$-4\kappa_{y_{1}}\frac{E_{fi}q_{i}}{b(2+q_{i})^{2}} + \varepsilon_{C_{1}}\frac{8E_{fi}q_{i}^{2}}{b^{2}(2+q_{i})^{3}} =$$

$$= 2v_{4i} + \frac{1}{H_{i}^{\frac{1}{m_{i}}}} \left[ \frac{8E_{fi}q_{i}^{2}}{b^{2}(2+q_{i})^{3}} v_{1i}^{\frac{1}{m_{i}}} - \frac{2E_{fi}q_{i}}{b(2+q_{i})} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{2i} + \left( -\frac{2E_{fi}q_{i}}{b(2+q_{i})^{2}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + \frac{1}{h_{i}} + \frac{1}{h_{i}} v_{2i}^{\frac{1-m_{i}}{m_{i}}} v_{2i} + \frac{1}{h_{i}} + \frac{1}{h_{i}} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-2m_{i}}{m_{i}}} v_{2i} + \frac{1}{h_{i}} + \frac{1}{h_{i}} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-2m_{i}}{m_{i}}} v_{2i} + \frac{1}{h_{i}} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{2i} + \frac{1}{h_{i}} \frac{1-m_{i}}{m_{i}^{2}} v_{2i} + \frac{1}{$$

$$-\kappa_{y_{1}} \frac{E_{fi}r_{i}\zeta_{i}}{2\theta_{i}^{2}} - \kappa_{z_{1}} \frac{2E_{fi}q_{i}}{b(2+q_{i})^{2}} =$$

$$=v_{5i} + \frac{1}{H_{i}^{\frac{1}{m_{i}}}} \left[ -\frac{2E_{fi}q_{i}}{b(2+q_{i})^{2}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} + \left( -\frac{E_{fi}r_{i}\zeta_{i}}{2\theta_{i}^{2}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} + \right)$$

$$+ \left( -\frac{E_{fi}r_{i}\zeta_{i}}{2\theta_{i}^{2}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right) v_{2i} + \left( -\frac{E_{fi}r_{i}\zeta_{i}}{2\theta_{i}^{2}} + \varepsilon_{C_{1}} \frac{E_{fi}r_{i}^{2}\zeta_{i}}{\theta_{i}^{3}} + \right) \right]$$

$$- 2\kappa_{z_{1}} \frac{E_{fi}r_{i}\zeta_{i}}{2\theta_{i}^{2}} + \varepsilon_{C_{1}} \frac{E_{fi}r_{i}^{2}\zeta_{i}}{\theta_{i}^{3}} = 2v_{6i} + \frac{1}{H_{i}^{\frac{1}{m_{i}}}} \left[ \frac{E_{fi}r_{i}^{2}\zeta_{i}}{\theta_{i}^{3}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} - \frac{E_{fi}r_{i}\zeta_{i}}{2\theta_{i}^{2}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} + \left( -\frac{E_{fi}r_{i}\zeta_{i}}{2\theta_{i}^{2}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} \right) \right]$$

$$+ E_{fi}\psi_{i} \frac{1-m_{i}}{m_{i}^{2}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} + \left( -\frac{E_{fi}r_{i}\zeta_{i}}{2\theta_{i}^{2}} \frac{1}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} \right) v_{3i}$$

$$+ E_{fi}\psi_{i} \frac{2}{m_{i}} v_{1i}^{\frac{1-m_{i}}{m_{i}}} v_{3i} \right]$$

where

$$\psi_{i} = \frac{1}{2} \left[ \frac{2}{2+q_{i}} + \frac{z_{1i+1} - z_{1i}}{z_{1i+1} - z_{1i} + r_{i} \left( z_{1ai} - z_{1i} \right)} \right]$$
(28)

$$\xi_i = z_{1i+1} - z_{1i} \tag{29}$$

$$\theta_i = z_{1i+1} - z_{1i} + r_i \left( z_{1ai} - z_{1i} \right)$$
(30)

Further, by substituting of (7) in equations (16) - (18), one obtains

$$N_{1} = b(z_{1i+1} - z_{1i}) \left( v_{1i} + v_{4i} \frac{b^{2}}{12} \right) + \frac{1}{2} v_{3i} b \left[ (z_{1i+1} - z_{1ai})^{2} - (z_{1i} - z_{1ai})^{2} \right] + (31) + \frac{1}{3} v_{6i} b \left[ (z_{1i+1} - z_{1ai})^{3} - (z_{1i} - z_{1ai})^{3} \right]$$

$$M_{y_{1}} = \frac{b}{2} \left( z_{1i+1}^{2} - z_{1i}^{2} \right) \left( v_{1i} - v_{3i} z_{1ai} + v_{4i} \frac{b^{2}}{12} + v_{6i} z_{1ai}^{2} \right) + \frac{1}{3} \left( z_{1i+1}^{3} - z_{1i}^{3} \right)$$
(32)  
$$\left( v_{3i} b - 2v_{6i} b z_{1ai} \right) + \frac{1}{4} v_{6i} b \left( z_{1i+1}^{4} - z_{1i}^{4} \right)$$

$$M_{z_{1}} = \frac{b^{3}}{12} v_{2i} (z_{1i+1} - z_{1i}) + \frac{b^{3}}{24} v_{5i} \left[ (z_{1i+1} - z_{1ai})^{2} - (z_{1i} - z_{1ai})^{2} \right]$$
(33)

where (Fig. 4)

$$N_1 = 0$$
 (34)

$$M_{y_1} = Fl_1 \tag{35}$$

$$M_{z_1} = 0$$
 (36)

In formulas (22) – (33),  $i = 1, 2, ..., n_1$ .

Equations (22) – (27) and (31) – (33) should be solved with respect to  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$ ,  $\kappa_{z_1}$ ,  $v_{1i}$ ,  $v_{2i}$ ,  $v_{3i}$ ,  $v_{4i}$ ,  $v_{5i}$  and  $v_{6i}$  by using the MATLAB computer program. Then,  $u_{01i}^*$  is obtained by substituting of (7) in (4).

Since the upper crack arm is free of stresses,

$$\mu_{02i}^* = 0, i = 1, 2, ..., n_2.$$
(37)

Formula (4) is applied also to calculate the complementary strain energy densities in the layers of the beam cross-section ahead of the crack front. For this purpose,  $\varepsilon_{C_1}$ ,  $\kappa_{y_1}$ ,  $\kappa_{z_1}$ ,  $v_{1i}$ ,  $v_{2i}$ ,  $v_{3i}$ ,  $v_{4i}$ ,  $v_{5i}$ ,  $v_{6i}$ ,  $n_1$ ,  $z_{1i}$ ,  $z_{1i+1}$ ,  $N_1$ ,  $M_{y_1}$  and  $M_{z_1}$  are replaced, respectively, with  $\varepsilon_{C_3}$ ,  $\kappa_{y_3}$ ,  $\kappa_{z_3}$ ,  $v_{r1i}$ ,  $v_{r2i}$ ,  $v_{r3i}$ ,  $v_{r4i}$ ,  $v_{r5i}$ ,  $v_{r6i}$ , n,  $z_{3i}$ ,  $z_{3i+1}$ ,  $N_3$ ,  $M_{y_3}$  and  $M_{z_3}$  in equations (22) – (27) and (31) – (33)

are the coordinates, where  $z_{3i}$ and  $z_{3i+1}$ respectively, of the upper and lower surfaces of the *i*th layer in the beam cross-section ahead of the crack front. Then, after solving of equations (22) - (27) and (31) – (33) with respect to  $\mathcal{E}_{C_3}$ ,  $\mathcal{K}_{y_3}$ ,  $\mathcal{K}_{z_3}$ ,  $\mathcal{V}_{r1i}$ ,  $\mathcal{V}_{r2i}$ ,  $v_{r3i}$ ,  $v_{r4i}$ ,  $v_{r5i}$  and  $v_{r6i}$ , the stress,  $\sigma_{ri}$ , is expressed by replacing of  $v_{1i}$ ,  $v_{2i}$ ,  $v_{3i}$ ,  $v_{4i}$ ,  $v_{5i}$ ,  $v_{6i}$ ,  $z_1$ ,  $y_1$  and  $z_{1ai}$ , respectively, with  $v_{r1i}$ ,  $v_{r2i}$ ,  $v_{r3i}$ ,  $v_{r4i}$ ,  $v_{r5i}$ ,  $v_{r6i}$ ,  $z_3$ ,  $y_3$  and  $z_{3ai}$  in (7) ( $z_{3ai}$  is calculated by replacing of  $z_{1i}$  and  $z_{1i+1}$ , respectively, with  $z_{3i}$  and  $z_{3i+1}$  in (6)). The complementary strain energy density is determined by substituting of  $\sigma_{ri}$ in (4).

After replacing of  $b_S$  with *b* and substituting of  $u_{01i}^*$ ,  $u_{02i}^*$  and  $u_{0i}^*$  in (3), the strain energy release rate, obtained by integrating of (3) with help of the MATLAB computer program, is doubled in view of the symmetry (Fig. 4).

In order to verify the solution to the strain energy release rate, the delamination fracture behaviour of the multilayered beam (Fig. 4) is analyzed also by using the method of *J*-integral [20]. The integration is carried-out along the integration contour,  $\Gamma$ , (Fig. 4). Since the upper crack arm is free of stresses, the *J*-integral value is zero in the upper crack arm. Therefore, the *J*-integral solution is written as

$$J = 2(J_{\Gamma_1} + J_{\Gamma_2})$$
(38)

where  $J_{\Gamma_1}$  and  $J_{\Gamma_2}$  are, respectively, the *J*-integral solutions in segments,  $\Gamma_1$  and  $\Gamma_2$ , of the integration contour. The segments,  $\Gamma_1$  and  $\Gamma_2$ , coincide, respectively, with the cross-sections of the lower crack arm and the beam ahead of the crack front (Fig. 4). The term in brackets in (38) is doubled in view of symmetry (Fig. 4).

In segment,  $\Gamma_1$ , of the integration contour, the *J*-integral is written as

$$J_{\Gamma_{1}} = \sum_{i=1}^{i=n_{1}} \int_{z_{1i}}^{z_{1i+1}} \begin{bmatrix} u_{01i} \cos \alpha_{\Gamma_{1}} - \\ - \left( p_{x_{i}} \frac{\partial u}{\partial x} + p_{y_{i}} \frac{\partial v}{\partial x} \right) \end{bmatrix} ds \quad (39)$$

where  $u_{0li}$  is the strain energy density in the *i*-th layer of lower crack arm behind the crack front,  $\alpha_{\Gamma_1}$  is the angle between the outwards normal vector to the contour of integration and the crack direction,  $p_{x_i}$  and  $p_{y_i}$  are the components of the stress vector, u and v are the components of the displacement vector with respect to the coordinate system xy, and ds is a differential element along the contour of integration. The components of (39) are determined as

$$p_{x_i} = -\sigma_i \tag{40}$$

$$p_{y_i} = 0 \tag{41}$$

$$ds = dz_1 \tag{42}$$

$$\cos \alpha_{\Gamma_1} = -1 \tag{43}$$

$$\frac{\partial u}{\partial x} = \varepsilon = \varepsilon_{C_1} + \kappa_{y_1} y_1 + \kappa_{z_1} z_1 \tag{44}$$

The strain energy density in the *i*-th layer of the lower crack arm is calculated by the formula [19]

$$u_{01i} = \frac{\sigma_i^2}{2E_i} + \frac{\sigma_i^{\frac{1+m_i}{m_i}}}{(1+m_i)H^{\frac{1}{m_i}}}$$
(45)

In formulae (40) and (45), the normal stress,  $\sigma_i$ , is determined by (7).

The *J*-integral in segment,  $\Gamma_2$ , is expressed as

$$J_{\Gamma_2} = \sum_{i=1}^{i=n} \int_{z_{3i}}^{z_{3i+1}} \left[ -\left( p_{xr_i} \frac{\partial u}{\partial x_r} + p_{yr_i} \frac{\partial v}{\partial x_r} \right) \right] ds_r \quad (46)$$

where

$$p_{xr_i} = \sigma_{ri} \tag{47}$$

$$p_{yr_i} = 0 \tag{48}$$

$$ds_r = -dz_3 \tag{49}$$

$$\cos \alpha_{\Gamma_2} = 1$$
 (50)

$$\frac{\partial u}{\partial x_r} = \varepsilon_{C_3} + \kappa_{y_3} y_3 + \kappa_{z_3} z_3 \tag{51}$$

$$u_{0i} = \frac{\sigma_{ri}^2}{2E_i} + \frac{\sigma_{ri}^{\frac{1+m_i}{m_i}}}{(1+m_i)H^{\frac{1}{m_i}}}$$
(52)

The average value of the *J*-integral along the delamination crack front is written as

$$J_{av} = \frac{1}{b} \int_{-\frac{b}{2}}^{\frac{b}{2}} J \, dy_1$$
(53)

By substituting of (39) and (46) in (53), one arrives at

$$J = \frac{2}{b} \sum_{i=1}^{i=n_1} \int_{-\frac{b}{2}}^{\frac{b}{2}} \int_{z_{1i+1}}^{z_{1i+1}} \left[ u_{01i} \cos \alpha_{\Gamma_1} - \left( p_{x_i} \frac{\partial u}{\partial x} + p_{y_i} \frac{\partial v}{\partial x} \right) \right] ds dy_1 + \frac{2}{b} \sum_{i=1}^{i=n} \int_{-\frac{b}{2}}^{\frac{b}{2}} \int_{z_{3i+1}}^{z_{3i+1}} \left[ u_{0i} \cos \alpha_{\Gamma_2} - \left( p_{xr_i} \frac{\partial u}{\partial x_r} + p_{yr_i} \frac{\partial v}{\partial x_r} \right) \right] ds_r dy_1$$

$$(54)$$

The integration in (54) should be performed by the MATLAB computer program. The fact that the *J*-integral values obtained by (54) exactly match the strain energy release rates calculated by (3) conforms the correctness of the delamination fracture analysis developed in the present paper. It should be mentioned that the delamination is analyzed also by keeping more than six members in the series of Taylor (5). The results are very close to these obtained by keeping the first six members (the difference is less than 3 %).

Effects of the material inhomogeneity and the nonlinear mechanical behaviour of the material on the delamination fracture are evaluated. For this purpose, calculations are carried-out by applying the solution to the strain energy release rate (3). The results obtained are presented in non-dimensional form by using the formula  $G_N = G / (E_{f1}b)$ . In order to elucidate the influence of the delamination crack location along the beam height, two three-layered four-point bending beam configurations are



Figure 6. Two three-layered four-point bending beam configurations with delamination crack located between (a) layers 2 and 3, and (b) layers 1 and 2

investigated (Fig. 6). In the first configuration, the delamination crack is located between layers 2 and 3 (Fig. 6a). A delamination crack between layers 1 and 2 is analyzed in the beam configuration shown in Fig. 6b. The thickness of each layer in both configurations is  $t_l$ . It is determined that b = 0.025 m,  $t_l = 0.006$  m,  $l_1 = 0.060$  m,  $l_2 = 0.100$  m and F = 10 N. The material inhomogeneity in width and thickness direction of layers is characterized by  $q_i$  and  $r_i$ .





In order to elucidate the influence of  $q_1$  on the delamination fracture behavior, the strain energy release rate in non-dimensional form is presented as a function of  $q_1$  in Fig. 7 for both three-layered beam configurations assuming that  $E_{f2} / E_{f1} = 0.6$ ,  $E_{f3} / E_{f1} = 0.8$ ,  $H_1 / E_{f1} = 0.7$ ,  $H_2 / H_1 = 1.4$ ,  $H_3 / H_1 = 0.5$ ,  $m_1 = m_2 = m_3 = 0.8$ ,  $q_2 = 0.6$ ,  $q_3 = 1.4$ ,  $r_1 = 2.5$ ,  $r_2 = 1.5$  and  $r_3 = 1.8$ . One can observe in Fig. 7 that the strain energy release rate increases with increasing of  $q_1$ . This behaviour is attributed to the fact that the beam stiffness increases with increasing of  $q_1$ . The curves in Fig. 7 indicate also that the strain energy release rate is lower when the delamination crack is located between layers 1 and 2 (Fig. 6b). This finding is explained with the fact that the thickness of the lower crack arm is higher when the delamination crack is between layers 1 and 2.

The effect of  $r_1$  on the delamination fracture behaviour is shown in Fig. 8 where the strain energy release rate is presented in non-dimensional form as a function of  $r_1$  at  $q_1 = 2.2$ .

The three-layered beam configuration shown in Fig. 6a is investigated. It can be observed in Fig. 8 that that strain energy release rate increases with increasing of  $r_1$ . The effect of the non-linear



Figure 8. The strain energy release rate in nondimensional form presented as a function  $r_1$  at (curve 1) non-linear mechanical behavior of the material and (curve 2) linear-elastic behaviour of the material for the three-layered four-point bending beam configuration with delamination crack located between layers 2 and 3 (refer to Fig. 6a)

mechanical behaviour of the material on the delamination fracture behaviour is elucidated too. For this purpose, the strain energy release rate is presented in Fig. 8 that is obtained assuming linearelastic behaviour of the inhomogeneous material in each layer of the beam configuration in nondimensional for comparison with the non-linear solution. It should be mentioned that the linear-elastic solution to the strain energy release rate is derived by substituting of  $H \rightarrow \infty$  in the non-linear solution (3) since at  $H \rightarrow \infty$  the Ramberg-Osgood stress-strain relation (1) transforms in the Hooke's law assuming that  $E_i$ is the modulus of elasticity of the inhomogeneous material in the *i*-th layer of the beam. The curves in Fig. 8 show that the non-linear mechanical behaviour of the material leads to increase of the strain energy release rate.

### 4 Conclusions

Delamination fracture behaviour of multilayered non-linear elastic beam structures has been investigated in this paper assuming that each layer exhibits smooth material inhomogeneity along the width as well as along thickness of the layer. The Ramberg-Osgood stress-strain relation is used for treating the non-linear mechanical behaviour of the inhomogeneous material. The delamination fracture behaviour is studied assuming that the modulus of elasticity varies continuously in the cross-section of each layer. A general approach for analysis of the strain energy release rate for delamination cracks in multilayered non-linear elastic inhomogeneous beams is developed in contrast to the previously papers which consider published particular multilayered beam configurations [15, 17]. The approach holds for beams of arbitrary cross-section in contrast to the previous papers which consider beams of rectangular cross-section. The approach can be applied at arbitrary distribution of the modulus of elasticity in both width and thickness directions of each layer of the beam (the previous papers consider particular distributions). The approach is valid for beams made of arbitrary number of layers. Each layer has individual thickness and material properties. The delamination crack is located arbitrary along the height of the multilayered beam. Besides, the approach can be used also when the law for distribution of the modulus of elasticity is different in each layer in contrast to the previous papers which are based on the assumption that the law is the same in each layer [15, 17]. It should be noted that the approach holds for non-linear elastic behaviour of the material. However, the approach can be applied also for elastic-plastic behaviour if the beam undergoes active deformation, i.e. if the external load increases only.

The general approach is applied for studying the delamination fracture behaviour of a multilayered beam configuration of rectangular cross-section. The beam is subjected to four-point bending. A delamination crack is located symmetrically with respect to the beam mid-span. The strain energy release rate is determined assuming that the continuous variation of the modulus of elasticity in the cross-section of each layer is described by a hyperbolic law. The delamination fracture behaviour is studied also by applying the *J*-integral method for verification of the solution to the strain energy release rate. Effects of material inhomogeneity in width and thickness directions of the layer, the delamination crack location along the beam height and the nonlinear mechanical behaviour of the inhomogeneous material on the delamination fracture behaviour are elucidated. The analysis reveals that the strain energy release rate increases with increasing of  $q_1$  and  $r_1$ . This finding is attributed to a decrease of the beam stiffness with increasing of  $q_1$  and  $r_1$ . It is also found that the strain energy release rate decreases with

increasing of the lower crack arm height. Concerning the effect of the non-linear mechanical behavior of the inhomogeneous material on the delamination fracture, the investigation shows that the strain energy release rate increases when the material nonlinearity is taken into account in the fracture analysis. The topic can be studied further in the future by assuming that not only the modulus of elasticity but also the other material properties,  $H_i$  and  $m_i$ , vary continuously in the cross-section of each layer of the beam. Also, the approach can be sophisticated further by considering the rheological behaviour (creep of strains and relaxation of stresses) and its influence on the delamination.

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